

Variably Saturated Flow and Transport—Sorbing Solute

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Introduction

In this example, water ponded in a ring on the ground moves into a relatively dry soil column and carries a chemical with it. As it moves through the variably saturated soil column, the chemical attaches to soil particles, slowing the solute transport relative to the water. Additionally the chemical concentrations decay from biodegradation in both the liquid and the solid phase. The inspiration for the problem comes from Ref. 1.



Figure 1: Geometry of the infiltration ring and soil column.

The example uses the Richards' Equation interface to define nonlinear relationships with retention and permeability properties according to van Genuchten (Ref. 2). In the Richards' Equation interface you can also define these material properties with Brooks and Corey's analytic permeability and retention formulas (Ref. 3) or by interpolation from experimental data.

Richards' equation accounts for changes in the fluid volume fraction with time, and also for changes in the storage related to variations in the pressure head according to Bear (Ref. 4). With the storage terms, the transport of diluted species in porous media equations in the Subsurface Flow Module also account explicitly for the time changes in liquid and air content.

Model Definition

The water moves from a ring on the ground into the subsurface. The 0.25-m radius ring ponds the water to a depth of 0.01 m but is open to the ground surface. Permeable soils exist to a depth of 1.3 m. The soil in the uppermost layer is slightly less permeable than the bottom one. The lower layer sits above relatively impermeable soil, so only a very small amount of leakage exits from the base. The flow is symmetric about the line r = 0. No flow crosses the surface outside the ring, and there is no flow across the line r = 1.25 m. The initial distribution of pressure heads is known.

The water in the ring contains a dissolved solute at a constant concentration, c_0 . The solute enters the ground with the water and moves through the subsurface by advection and dispersion. Additionally, the solute adsorbs or attaches to solid surfaces, which reduces the aqueous concentrations and also slows solute movement relative to the water. Microbial degradation also reduces both the liquid-phase and solid-phase concentrations. The sorption and the biodegradation are linearly proportional to aqueous concentrations. The fluid in the ring is the only chemical source, and the solute is free to leave the soil column with the fluid flux. Initially the soil is free of the solute. You track its transport for five days.

FLUID FLOW

Richards' equation governs the saturated-unsaturated flow of water in the soil. The soil air is open to the atmosphere, so you can assume that pressure changes in air do not affect the flow and use Richards' equation here for single-phase flow. Given by Ref. 4, Richards' equation in pressure head reads

$$(C + \text{Se}S)\frac{\partial H_p}{\partial t} + \nabla \cdot (-K\nabla (H_p + D)) = 0$$

where *C* denotes specific moisture capacity (m^{-1}) ; Se is the effective saturation of the soil (dimensionless); *S* is a storage coefficient (m^{-1}) ; H_p is the pressure head (m), which is proportional to the dependent variable, *p* (Pa); *t* is time; *K* equals the hydraulic conductivity (m/s); *D* is the direction (typically, the *z* direction) that represents vertical elevation (m).

To be able to combine boundary conditions and sources with the Darcy's Law formulation, COMSOL Multiphysics converts Richards' equation to SI units and solves for the pressure (SI unit: Pa). Hydraulic head, H, pressure head, H_p , and elevation D are related to pressure p as

$$H_p = \frac{p}{\rho g}; \qquad H = H_p + D$$

Also, the permeability κ (SI unit: $1/m^2$) and hydraulic conductivity *K* (SI unit: m/s) are related to the viscosity μ (SI unit: Pa s) and density ρ (SI unit: kg/m³) of the fluid, and the acceleration of gravity *g* (SI unit: m/s²) by

$$\frac{\kappa}{\mu} = \frac{K}{\rho g}$$

In this problem, $S = (\theta_s - \theta_r)/(1 \text{ m} \cdot \rho g)$ where θ_s and θ_r denote the volume fraction of fluid at saturation and after drainage, respectively. For more details see The Richards' Equation Interface in the *Subsurface Flow Module User's Guide*.

With variably saturated flow, fluid moves through but may or may not completely fill the pores in the soil, and θ denotes the volume fraction of fluid within the soil. The coefficients C, Se, and K vary with the pressure head, H_p , and with θ , making Richards' equation nonlinear. The specific moisture capacity, C, relates variations in soil moisture to pressure head as $C = \partial \theta / \partial H_p$. In the governing equation, C defines storage changes produced by varying fluid content because $C\partial H_p / \partial t = \partial \theta / \partial t$. Because C, the first term in the time coefficient, goes to zero at saturation, time change in storage relates to compression of the aquifer and water under saturated conditions. The saturated storage comes about with the effective saturation, as represented by the second term in the time-coefficient. Furthermore, K is a function that defines how readily the porous media transmits fluid. The relative permeability of the soil, k_r , increases with fluid content giving $K = K_s k_r$, where K_s (m/d) is the constant hydraulic conductivity at saturation.

This example uses predefined interfaces for van Genuchten formulas (Ref. 2) to represent how the four retention and permeability properties— θ , *C*, Se, and $k_r = K/K_s$ —vary with the solution H_p . The van Genuchten expressions read as follows:

$$\theta = \begin{cases} \theta_{\rm r} + \operatorname{Se}(\theta_{\rm s} - \theta_{\rm r}) & H_p < 0\\ \theta_s & H_p \ge 0 \end{cases}$$

Se =
$$\begin{cases} \frac{1}{\left[1 + \left|\alpha H_p\right|^n\right]^m} & H_p < 0\\ 1 & H_p \ge 0 \end{cases}$$

$$C = \begin{cases} \frac{\alpha m}{1-m} (\theta_{s} - \theta_{r}) \operatorname{Se}^{\frac{1}{m}} \left(1 - \operatorname{Se}^{\frac{1}{m}}\right)^{m} & H_{p} < 0\\ 0 & H_{p} \ge 0 \end{cases}$$
$$k_{r} = \begin{cases} \operatorname{Se}^{l} \left[1 - \left(1 - \operatorname{Se}^{\frac{1}{m}}\right)^{m}\right]^{2} & H_{p} < 0\\ 1 & H_{p} \ge 0 \end{cases}$$

Here α , *n*, *m*, and *l* are constants that specify the type of medium, with m = 1 - 1/n. In the equations, the system reaches saturation when fluid pressure is atmospheric (that is, $H_p = 0$). When the soil fully saturates, the four parameters reach constant values.

Boundary Conditions and Initial Conditions

The problem statement records all the boundary conditions you need for this model. The level of water in the ring is known at 0.01 m, giving a Dirichlet constraint on pressure head. Approximate the small leak from the base, N_0 , as $0.01K_s$. With no flow crossing the surface outside of the pressure ring or the vertical walls, the following expressions summarize the boundary conditions:

$H_p = H_{p0}$	$\partial \Omega$ Ring
$\mathbf{n} \cdot \mathbf{u} = 0$	$\partial \Omega$ Surface
$\mathbf{n} \cdot \mathbf{u} = 0$	$\partial \Omega$ Sides
$\mathbf{n} \cdot \mathbf{u} = 0$	$\partial \Omega$ Symmetry
$\mathbf{n} \cdot \rho \mathbf{u} = N_0$	$\partial \Omega$ Base

In these expressions, \mathbf{n} is the unit vector normal to the bounding surface, and \mathbf{u} is Darcy's velocity field. The initial conditions specify the pressure head in the modeling domain as

$$H_p = \begin{cases} -(z+1.2 \text{ m}) - 0.2(z+0.4 \text{ m}) & \text{Upper layer} \\ -(z+1.2 \text{ m}) & \text{Lower layer} \end{cases}$$

TRANSPORT OF DILUTED SPECIES IN POROUS MEDIA

Groundwater flow and solute transport are linked by fluid velocities. With the form of the transport equation that follows, the fluid velocities need to come from Darcy's law:

$$\mathbf{u} = K_{\mathrm{s}}k_{\mathrm{r}}\nabla(H_{\mathrm{n}} + D)$$

In this expression, **u** is Darcy's velocity field (SI unit: m/s).

The equation that governs advection, dispersion, sorption, and decay of solutes in groundwater is

$$\frac{\partial}{\partial t}(\theta c) + \frac{\partial}{\partial t}(\rho_{\rm b}c_{\rm P}) + \mathbf{u} \cdot \nabla c + \nabla \cdot [-\theta D_{\rm L} \nabla c] = \Sigma R_{\rm L} + \Sigma R_{\rm P} + S_{\rm c}$$

It describes time rate of change in two terms: *c* denotes dissolved concentration (mol/m³), and *c*_P is the mass of adsorbed contaminant per dry unit weight of solid (mg/kg). Further, θ denotes the volume fluid fraction (porosity), and ρ_b is the bulk density (kg/m³). Because ρ_b amounts to the dried solid mass per bulk volume of the solids and pores together, the term $\rho_b c_P$ gives solute mass attached to the soil as a concentration. In the equation, D_L is the hydrodynamic dispersion tensor (m²/d); R_L represents reactions in water (mol/(m³·d)); and R_P equals reactions involving solutes attached to soil particles (mol/(m³·d)). Finally, S_c is solute added per unit volume of soil per unit time (mol/(m³·d)).

It is far more convenient to solve the above equation only for dissolved concentration. This requires expanding the left-hand side to

$$\frac{\partial}{\partial t}(\theta c) + \frac{\partial}{\partial t}(\rho_{\rm b} c_{\rm P}) = \theta \frac{\partial c}{\partial t} + c \frac{\partial \theta}{\partial t} + \rho_{\rm b} \frac{\partial c_{\rm P}}{\partial c} \frac{\partial c}{\partial t}$$

and inserting a few definitions.

In this problem, solute mass per solid mass, $c_{\rm P}$, relates to dissolved concentration, c, through a linear isotherm or partition coefficient $k_{\rm P}$ (m³/kg) where $c_{\rm P} = k_{\rm P}c$. Because the relationship is linear, the derivative is $k_{\rm P} = \partial c_{\rm P}/\partial c$. Making those substitutions gives the form of the solute transport problem you solve:

$$(\theta + \rho_{\rm b}k_{\rm P})\frac{\partial c}{\partial t} + c\frac{\partial \theta}{\partial t} + \mathbf{u} \cdot \nabla c + \nabla \cdot (-\theta D_{\rm L}\nabla c) = \theta\phi_{\rm L}c + \rho_{\rm b}k_{\rm P}\phi_{\rm P}c + S_{\rm c}$$

In the equation, ϕ_L and ϕ_P denote the decay rates (d^{-1}) for the dissolved and adsorbed solute concentrations, respectively.

Select the **Time change in pressure head** option for **Fluid fraction time change** in the **Saturation** settings for the Partially Saturated Porous Media feature to employ results from the flow equation in the solute-transport model:

$$c\frac{\partial \theta}{\partial t} = cC\frac{\partial H_p}{\partial t}$$

Note that COMSOL Multiphysics solves for pressure, p, and converts to H_p based on the fluid weight.

The hydrodynamic dispersion tensor, $D_{\rm L}$, describes mechanical spreading from groundwater movement in addition to chemical diffusion:

$$\begin{split} \theta D_{\mathrm{L}ii} &= \alpha_1 \frac{u_i^2}{|\mathbf{u}|} + \alpha_2 \frac{u_j^2}{|\mathbf{u}|} + \theta \frac{D_{\mathrm{m}}}{\tau_{\mathrm{L}}} \\ \theta D_{\mathrm{L}ij} &= \theta D_{\mathrm{L}ij} = (\alpha_1 - \alpha_2) \frac{u_i u_j}{|\mathbf{u}|} \end{split}$$

where $D_{\text{L}ii}$ are the diagonal entries in the dispersion tensor; $D_{\text{L}ij}$ are the cross terms; α is the dispersivity (m); the subscripts "1" and "2" denote longitudinal and transverse dispersivities, respectively; D_{m} denotes the coefficient of molecular diffusion (m²/d); and τ_{L} is a tortuosity factor that reduces impacts of molecular diffusion for porous media relative to free water. Here $\tau_{\text{L}} = \theta^{-7/3} \theta_{\text{s}}^{2}$.

Boundary Conditions and Initial Conditions

The boundary and initial conditions in the sorbing-solute problem are straightforward. The solute enters only with the water from the ring at a concentration c_0 . The solute is free to leave, but there is only minimal leakage from the lower boundary and no flow from the sides. Transport is symmetric about the line r = 0. The boundary conditions in this problem are:

$c = c_0$	$\partial \Omega$ Ring
$\mathbf{n} \cdot (-\theta D_{\rm L} \nabla c) = 0$	$\partial \Omega$ Surface
$\mathbf{n} \cdot (-\theta D_{\rm L} \nabla c) = 0$	$\partial \Omega$ Sides
$\mathbf{n} \cdot (-\theta D_{\rm L} \nabla c + \mathbf{u} c) = 0$	$\partial \Omega$ Symmetry
$\mathbf{n} \cdot (-\theta D_{\rm L} \nabla c) = 0$	$\partial \Omega$ Base

where \mathbf{n} is the unit vector normal to the boundary. Because the soil is pristine at the start of the experiment, the initial condition is one of zero concentration.

MODEL DATA

The following table provides data for the fluid-flow model:

VARIABLE	UNIT	DESCRIPTION	UPPER LAYER	LOWER LAYER
$K_{ m s}$	m/d	Saturated hydraulic conductivity	0.298	0.454
$\theta_{\mathbf{s}}$		Porosity/void fraction	0.399	0.339
$\theta_{\rm r}$		Residual saturation	0.001	0.001
α	m ⁻¹	alpha parameter	1.74	1.39
n		n parameter	1.38	1.60
m		m parameter	I-I/n	I-I/n
l		Pore connectivity parameter	n/a	
H_{p0}	m	Pressure head in ring	0.01	
$H_{p,\mathrm{init}}$	m	Initial pressure head	-(z+1.2) -0.2(z+0.4)	-(z+1.2)

The inputs needed for the solute-transport model are:

VARIABLE	UNITS	DESCRIPTION	VALUE
ρ_{b}	kg/m ³	Bulk density	1400
k_p	m ³ /kg	Partition coefficient	0.0001
D_m	m²/d	Coefficient of molecular diffusion	0.00374
α_r	m	Longitudinal dispersivity	0.005
α_z	m	Transverse dispersivity	0.001
ϕ_L	d-I	Decay rate in liquid	0.05
ϕ_P	d-I	Decay rate on solid	0.01
c_0	mol/m ³	Solute concentration in ring	1.0

Results and Discussion

Figure 2 and Figure 3 give the solution to the fluid-flow problem at 0.3 days and 1 day, respectively. The images show effective saturation (surface plot), pressure head (contours), and velocities (arrows). The figures illustrate the soil wetting with time. As the arrows indicate, the velocities just below the ring are high relative to the remainder of the soil column.



Figure 2: Estimates of effective saturation (surface plot), pressure head (contours), and velocity (arrows) in variably saturated soil after 0.3 days.



Figure 3: Estimates of effective saturation (surface plot), pressure head (contours), and velocity (arrows) in variably saturated soil after 1 day.

Figure 4 and Figure 5 give the concentrations for 0.3 days and 1 day, respectively, along with the retardation factor. They illustrate how the solute concentrations (surface plot) enter and move through the soil. Because the retardation factor depends on soil moisture, its value varies with the solution.



Figure 4: Solution for dissolved concentrations (surface plot) and retardation factor (contours) at 0.3 days.



Figure 5: Solution for dissolved concentrations (surface plot) and retardation factor (contours) at 1 day.

Figure 6 shows an image of the retardation factor at the end of the simulation time interval. For variably saturated solute transport, the retardation factor changes with time. As shown in this image, the process of sorption has the greatest potential to slow the contaminant where the soils are relatively dry. The retardation coefficient here ranges from roughly 1.35 to 1.55, and the solute moves at approximately the velocity of the groundwater.



Figure 6: Snapshot of the retardation factor (surface and contours) at 5 days.

Notes About the COMSOL Implementation

This model makes use of the **Infinite Element Domain** feature. It performs a coordinate scaling to the selected domain such that boundary conditions on the outside of the infinite element layer are effectively applied at a very large distance. Therefore unwanted effects of artificial boundary conditions on the region of interest are suppressed. This allows to model details in an area which is actually very large or infinite.

References

1. J. Simunek, T. Vogel, and M.Th. van Genuchten, "The SWMS_2D code for simulating water flow and solute transport in two-dimensional variably saturated media," ver. 1.1., *Research Report No. 132*, U.S. Salinity Laboratory, USDA, 1994.

2. M.Th. van Genuchten, "A closed-form equation for predicting the hydraulic of conductivity of unsaturated soils," *Soil Sci. Soc. Am. J.*, vol. 44, pp. 892–898, 1980.

3. R.H. Brooks and A.T. Corey, "Properties of porous media affecting fluid flow," *J. Irrig. Drainage Div.*, ASCE Proc. 72(IR2), pp. 61–88, 1966.

4. J. Bear, Hydraulics of Groundwater, McGraw-Hill, 1978.

Application Library path: Subsurface_Flow_Module/Solute_Transport/ sorbing_solute

Modeling Instructions

From the File menu, choose New.

NEW

In the New window, click Model Wizard.

MODEL WIZARD

- I In the Model Wizard window, click 2D Axisymmetric.
- 2 In the Select Physics tree, select Fluid Flow>Porous Media and Subsurface Flow> Richards' Equation (dl).
- 3 Click Add.
- 4 In the Select Physics tree, select Chemical Species Transport> Transport of Diluted Species in Porous Media (tds).
- 5 Click Add.
- 6 Click Study.
- 7 In the Select Study tree, select Preset Studies for Selected Physics Interfaces> Time Dependent.
- 8 Click Done.

GLOBAL DEFINITIONS

Load the parameters from file.

Parameters

- I On the Home toolbar, click Parameters.
- 2 In the Settings window for Parameters, locate the Parameters section.
- **3** Click Load from File.

4 Browse to the model's Application Libraries folder and double-click the file sorbing_solute_parameters.txt.

GEOMETRY I

The modeling domain is made up of the two permeable soil layers, each of which is represented by a rectangular domain in 2D axisymmetry.

Rectangle 1 (r1)

- I On the Geometry toolbar, click Primitives and choose Rectangle.
- 2 In the Settings window for Rectangle, locate the Size and Shape section.
- 3 In the Width text field, type 1.5.
- 4 In the **Height** text field, type 0.9.
- 5 Locate the Position section. In the z text field, type -1.3.
- 6 Click to expand the Layers section. Select the Layers to the right check box.
- 7 Clear the Layers on bottom check box.
- 8 In the table, enter the following settings:

Layer name	Thickness (m)	
Layer 1	0.25	

This additional layer to the right will later be used to define an **Infinite Element Domain**. Read more about it in the Notes About the COMSOL Implementation section. Proceed with the second soil layer.

Rectangle 2 (r2)

- I On the Geometry toolbar, click Primitives and choose Rectangle.
- 2 In the Settings window for Rectangle, locate the Size and Shape section.
- 3 In the Width text field, type 1.5.
- 4 In the **Height** text field, type 0.4.
- 5 Locate the **Position** section. In the **z** text field, type -0.4.
- 6 Locate the Layers section. Select the Layers to the right check box.
- 7 Clear the Layers on bottom check box.

8 In the table, enter the following settings:

Layer name	Thickness (m)	
Layer 1	0.25	

To finish the model geometry, add a point on the top boundary marking the pond's outer rim.

Point I (ptl)

- I On the Geometry toolbar, click Primitives and choose Point.
- 2 In the Settings window for Point, locate the Point section.
- **3** In the **r** text field, type **0.25**.
- 4 Click Build All Objects.
- **5** Click the **Zoom Extents** button on the **Graphics** toolbar.



Now, define the Infinite Element Domain.

DEFINITIONS

Infinite Element Domain 1 (ie1)

- I On the **Definitions** toolbar, click **Infinite Element Domain**.
- 2 Select Domains 3 and 4 only.

- 3 In the Settings window for Infinite Element Domain, locate the Geometry section.
- 4 From the Type list, choose Cylindrical.

RICHARDS' EQUATION (DL)

Begin by specifying the properties for the bottom soil layer in the default Richards' Equation Model node, then duplicate this node and modify the domain selection and properties to match the top layer.

Richards' Equation Model 1

- I In the Model Builder window, under Component I (compl)>Richards' Equation (dl) click Richards' Equation Model I.
- 2 In the Settings window for Richards' Equation Model, locate the Fluid Properties section.
- **3** From the ρ list, choose **User defined**. In the associated text field, type rho.
- 4 Locate the Matrix Properties section. From the Permeability model list, choose Hydraulic conductivity.
- **5** In the K_s text field, type Ks_1.
- **6** In the θ_s text field, type thetas_1.
- 7 In the θ_r text field, type thetar_1.
- 8 Locate the Storage Model section. From the Storage list, choose User defined. In the S text field, type Ss_1.
- 9 Locate the Retention Model section. In the α text field, type alpha_1.
- **IO** In the *n* text field, type n_1.

Richards' Equation Model 2

- I Right-click Component I (comp1)>Richards' Equation (dl)>Richards' Equation Model I and choose Duplicate.
- **2** Select Domains 2 and 4 only.
- 3 In the Settings window for Richards' Equation Model, locate the Matrix Properties section.
- **4** In the K_s text field, type Ks_2.
- **5** In the θ_s text field, type thetas_2.
- **6** In the θ_r text field, type thetar_2.
- 7 Locate the Storage Model section. In the S text field, type Ss_2.
- **8** Locate the **Retention Model** section. In the α text field, type alpha_2.
- **9** In the *n* text field, type n_2.

Initial Values 1

- I In the Model Builder window, under Component I (compl)>Richards' Equation (dl) click Initial Values I.
- 2 In the Settings window for Initial Values, locate the Initial Values section.
- 3 Click the Pressure head button.
- 4 In the H_p text field, type (z+1.2).

Initial Values 2

- I Right-click Component I (comp1)>Richards' Equation (dl)>Initial Values I and choose Duplicate.
- 2 Select Domains 2 and 4 only.
- 3 In the Settings window for Initial Values, locate the Initial Values section.
- 4 In the H_p text field, type (z+1.2)-0.2*(z+0.4).

Pressure Head I

- I On the Physics toolbar, click Boundaries and choose Pressure Head.
- **2** Select Boundary 5 only.
- 3 In the Settings window for Pressure Head, locate the Pressure Head section.
- **4** In the H_{p0} text field, type Hp0.

Pervious Layer 1

- I On the Physics toolbar, click Boundaries and choose Mass Flux.
- 2 On the Physics toolbar, click Boundaries and choose Pervious Layer.
- **3** Select Boundaries 2 and 8 only.
- 4 In the Settings window for Pervious Layer, locate the Pervious Layer section.
- **5** In the $H_{\rm b}$ text field, type -2.
- 6 In the $R_{\rm b}$ text field, type 1/5[d].

Gravity I

- I In the Model Builder window, under Component I (compl)>Richards' Equation (dl) click Gravity I.
- 2 In the Settings window for Gravity, locate the Gravity section.
- 3 From the Specify list, choose Elevation.

TRANSPORT OF DILUTED SPECIES IN POROUS MEDIA (TDS)

Now, set up the transport equation for an unsaturated porous medium, accounting for dispersion and adsorption

On the Physics toolbar, click Richards' Equation (dl) and choose Transport of Diluted Species in Porous Media (tds).

- I In the Model Builder window, under Component I (comp1) click Transport of Diluted Species in Porous Media (tds).
- 2 In the Settings window for Transport of Diluted Species in Porous Media, locate the Transport Mechanisms section.
- 3 Find the Porous media transport subsection. Select the Dispersion check box.

Partially Saturated Porous Media I

- I On the Physics toolbar, click Domains and choose Partially Saturated Porous Media.
- 2 In the Settings window for Partially Saturated Porous Media, locate the Domain Selection section.
- 3 From the Selection list, choose All domains.
- 4 Locate the Convection section. From the **u** list, choose Darcy's velocity field (dl).
- 5 Locate the Matrix Properties section. From the ϵ_p list, choose User defined. In the associated text field, type dl.thetas.

This corresponds to the saturated liquid volume fraction, defined in the **Richards' Equation** interface.

- **6** From the ρ list, choose **User defined**. In the associated text field, type rhob.
- 7 Locate the Saturation section. From the list, choose Liquid volume fraction.
- **8** In the θ text field, type dl.theta.

This corresponds to the liquid volume fraction.

- 9 From the Fluid fraction time change list, choose Time change in pressure head.
- **IO** From the $dH_{p/dt}$ list, choose **Time change in pressure head (dl)**.
- II In the $C_{\rm m}$ text field, type dl.Cm.
- 12 Locate the Diffusion section. In the $D_{L,c}$ text field, type D1.
- 13 Locate the Dispersion section. From the Dispersion tensor list, choose Dispersivity.
- **I4** From the **Dispersivity model** list, choose **Transverse isotropic**.
- **I5** In the α table, enter the following settings:

alphar

alphaz

Add adsorption as a subnode to the Partially Saturated Porous Media node.

Adsorption 1

- I On the Physics toolbar, click Attributes and choose Adsorption.
- 2 In the Settings window for Adsorption, locate the Adsorption section.
- 3 From the Species c list, choose User defined.
- **4** In the $k_{\text{P. c}}$ text field, type kp.

Reactions I

- I On the Physics toolbar, click Domains and choose Reactions.
- 2 In the Settings window for Reactions, locate the Domain Selection section.
- 3 From the Selection list, choose All domains.
- 4 Locate the **Reaction Rates** section. In the R_c text field, type (phip*kp*rhob-phil* dl.theta)*c.

Outflow I

- I On the Physics toolbar, click Boundaries and choose Outflow.
- 2 Select Boundaries 2, 8, 12, and 13 only.

Concentration 1

- I On the Physics toolbar, click Boundaries and choose Concentration.
- **2** Select Boundary 5 only.
- 3 In the Settings window for Concentration, locate the Concentration section.
- 4 Select the **Species c** check box.
- **5** In the $c_{0,c}$ text field, type c0.

MESH I

Using a mapped mesh is a good idea for this geometry. It uses less mesh elements while keeping the accuracy compared to using a triangular mesh with the same mesh size.

- I In the Model Builder window, under Component I (compl) click Mesh I.
- 2 In the Settings window for Mesh, locate the Mesh Settings section.
- 3 From the Element size list, choose Finer.

Size 1

- I Right-click Component I (compl)>Mesh I and choose Mapped.
- 2 In the Model Builder window, under Component I (compl)>Mesh I right-click Mapped I and choose Size.
- 3 In the Settings window for Size, locate the Geometric Entity Selection section.

- 4 From the Geometric entity level list, choose Domain.
- **5** Select Domain 2 only.
- 6 Locate the **Element Size** section. Click the **Custom** button.
- 7 Locate the Element Size Parameters section. Select the Maximum element size check box.
- 8 In the associated text field, type 0.02.
- 9 In the Model Builder window, click Mesh I.

10 In the Settings window for Mesh, click Build All.



STUDY I

Step 1: Time Dependent

- I In the Settings window for Time Dependent, locate the Study Settings section.
- 2 From the Time unit list, choose d.
- 3 In the Times text field, type range(0,0.1,0.9) range(1,1,5).
- **4** On the **Home** toolbar, click **Compute**.

RESULTS

Data Sets

Flownet, pressure and concentration plots are created per default. Pressure and concentration are also visualized on a revolved 3D geometry. Visualizing the results on the infinite element domains doesn't add value to the plots. Focus on the region close to the source and therefore hide the infinite element domains from the plots with the following steps.

Study I/Solution I (soll)

In the Model Builder window, expand the Results>Data Sets node, then click Study I/ Solution I (soll).

Selection

- I On the **Results** toolbar, click **Selection**.
- 2 In the Settings window for Selection, locate the Geometric Entity Selection section.
- 3 From the Geometric entity level list, choose Domain.
- 4 Select Domains 1 and 2 only.

Pressure (dl)

The second default plot group contains a surface plot of the pressure distribution. Modify it to show the effective saturation, pressure head, and velocity field at different times.

- I In the Model Builder window, under Results click Pressure (dl).
- 2 In the Settings window for 2D Plot Group, type Effective saturation in the Label text field.
- 3 Locate the Data section. From the Time (d) list, choose 0.3.

Surface

- I In the Model Builder window, expand the Results>Effective saturation node, then click Surface.
- In the Settings window for Surface, click Replace Expression in the upper-right corner of the Expression section. From the menu, choose Model>Component I>
 Richards' Equation>dl.Se Effective saturation.
- 3 Click to expand the Title section. From the Title type list, choose Custom.
- 4 Find the Type and data subsection. Clear the Unit check box.

Contour I

I In the Model Builder window, under Results right-click Effective saturation and choose Contour.

- In the Settings window for Contour, click Replace Expression in the upper-right corner of the Expression section. From the menu, choose Model>Component I>
 Richards' Equation>dl.Hp Pressure head.
- 3 Locate the Levels section. In the Total levels text field, type 10.
- 4 Locate the Coloring and Style section. From the Coloring list, choose Uniform.
- 5 From the Color list, choose Black.
- 6 Clear the Color legend check box.
- 7 Click to expand the Quality section. From the Resolution list, choose Finer.

Arrow Surface 1

- I Right-click Effective saturation and choose Arrow Surface.
- 2 In the Settings window for Arrow Surface, locate the Coloring and Style section.
- 3 From the Color list, choose Black.
- 4 Locate the **Arrow Positioning** section. Find the **R grid points** subsection. In the **Points** text field, type 20.
- 5 Find the Z grid points subsection. In the Points text field, type 20.
- 6 On the Effective saturation toolbar, click Plot.

Compare the plot in the Graphics window with that in Figure 2.

Effective saturation

- I In the Model Builder window, under Results click Effective saturation.
- 2 In the Settings window for 2D Plot Group, locate the Data section.
- **3** From the **Time (d)** list, choose **I**.
- **4** On the **Effective saturation** toolbar, click **Plot**.

Compare with the plot in Figure 3.

The third default plot shows the solute concentration. Follow the steps below to reproduce the plots shown in Figure 4 and Figure 5.

Concentration (tds)

- I In the Model Builder window, under Results click Concentration (tds).
- 2 In the Settings window for 2D Plot Group, locate the Data section.
- **3** From the **Time (d)** list, choose **0.3**.

Contour I

I Right-click Results>Concentration (tds) and choose Contour.

- 2 In the Settings window for Contour, click Replace Expression in the upper-right corner of the Expression section. From the menu, choose Model>Component I> Transport of Diluted Species in Porous Media>tds.RF_c - Retardation factor.
- 3 Click to expand the Title section. From the Title type list, choose Custom.
- 4 Find the Type and data subsection. Clear the Unit check box.
- **5** Locate the **Levels** section. In the **Total levels** text field, type **10**.
- 6 Locate the Coloring and Style section. From the Coloring list, choose Uniform.
- 7 From the Color list, choose White.
- 8 Clear the **Color legend** check box.
- 9 Locate the Quality section. From the Resolution list, choose Fine.
- 10 On the Concentration (tds) toolbar, click Plot.

Compare the result with that in Figure 4.

Concentration (tds)

- I In the Model Builder window, under Results click Concentration (tds).
- 2 In the Settings window for 2D Plot Group, locate the Data section.
- 3 From the Time (d) list, choose I.
- 4 On the Concentration (tds) toolbar, click Plot.

Compare with Figure 5.

Finally, plot the retardation factor after 5 days (Figure 6).

Concentration (tds) 2

- I Right-click Results>Concentration (tds) and choose Duplicate.
- 2 In the Settings window for 2D Plot Group, type Retardation factor in the Label text field.
- 3 Locate the Data section. From the Time (d) list, choose 5.

Surface 1

- I In the Model Builder window, expand the Results>Retardation factor node, then click Surface 1.
- In the Settings window for Surface, click Replace Expression in the upper-right corner of the Expression section. From the menu, choose Model>Component I>
 Transport of Diluted Species in Porous Media>tds.RF_c Retardation factor.
- 3 Locate the Title section. From the Title type list, choose Custom.
- 4 Find the Type and data subsection. Clear the Unit check box.

Contour I

- I In the Model Builder window, under Results>Retardation factor click Contour I.
- 2 In the Settings window for Contour, locate the Coloring and Style section.
- **3** From the **Color** list, choose **Black**.
- 4 On the Retardation factor toolbar, click Plot.

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